HILTI OR EQUIVANLENT

1. EXPOXY SHALL BE HILTI HIT-RE 500-SD ADHESIVE AS MANUFACTURED BY HILTI, INC., 5400 S. 122nd EAST AVENUE, TULSA, OK 74146. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND I.C.C. REPORT NO. ESR-2322.

2. ULTIMATE TENSION VALUES SHALL BE AS FOLLOWS:

BAR SIZE	BOLT SIZE	MIN. EMBEDMENT	TENSION LOAD (POUNDS)
#4	1/2"	4"	1700
#5	5/8"	5"	1725
#6	3/4"	7"	2500

3. PLACEMENT GUIDELINES FOR ABOVE VALUES IN ITEM 2 REQUIRE THE FOLLOWING CONDITIONS.
   - TABLE VALUES ARE BASED ON fc = 3000 PSI
   - 6" MIN. EDGE DISTANCE & 12" MIN. BOLT/BAR SPACING
   - HOLES DRILLED WITH A HAMMER DRILL AND CARBIDE BIT
   - BIT DIAMETER EQUALS (BAR DIAMETER) + 1/8"
   - HOLES SHALL BE DRY OR WATER-SATURATED, BUT NOT WATER FILLED
   - MAX. LONG TERM TEMPS OF 110° F OR LESS & MAX. SHORT TERM OF 162° F OR LESS
   - ANY SEISMIC DESIGN CATEGORY PER I.B.C. 2009
   - A.S.T.M. A 615 GRADE 60 REBAR
   - A.S.T.M. A 193 GRADE B7 THREADED RODS
   - A.C.I. "CRACKED" CONCRETE CONDITION IS SUFFICIENT
   - DEPTH/THICKNESS OF CONCRETE BEING ATTACHED TO MUST BE AT LEAST 1 1/2" GREATER THAN THE EMBEDMENT DEPTH.

4. WHEN INSTALLING EPOXIED REBAR/BOLTS IN EXISTING CONCRETE, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS. MAINTAIN A MINIMUM CLEARANCE OF ONE-INCH BETWEEN THE EXISTING REINFORCEMENT AND THE EPOXIED REBAR/BOLT.

5. ANY REBAR/BOLTS SHOWN ON THE APPROVED PLANS AS BEING EPOXIED, REQUIRES SPECIAL INSPECTION IN ACCORDANCE WITH SECTION 4.4 IN THE I.C.C. REPORT. SPECIAL INSPECTION SHALL BE BY AN APPROVED TESTING AND INSPECTION AGENCY. ANY ITEMS THAT REQUIRE EPOXY BUT ARE NOT SPECIFICALLY SHOWN AS BEING EPOXIED ON THE APPROVED PLANS MUST BE APPROVED BY THE STRUCTURAL ENGINEER AND BUILDING DEPARTMENT PRIOR TO INSTALLATION.

B2 HILTI HIT-RE 500-SD EPOXY NOTES IN CONC.

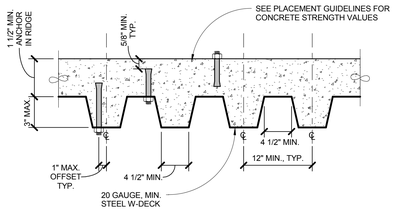
SS101 NOT TO SCALE

HILTI OR EQUIVALENT

1. EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ AS MANUFACTURED BY HILTI, INC., 5400 SOUTH 122ND EAST AVENUE, TULSA, OKLAHOMA 74146. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND I.C.C. REPORT NO. ESR-1917.

2. ULTIMATE TENSION VALUES SHALL BE AS FOLLOWS:

BOLT SIZE	MIN. EMBED.	EDGE DIST.	CONC. THKS.	TENSION LOAD	TORQUE TEST
3/8"	2"	4 1/2"	4"	1000#	25 #-FT
1/2"	3 1/4"	7 3/4"	6"	1850#	40 #-FT
5/8"	4"	9"	6"	3300#	60 #-FT
3/4"	4 3/4"	10 1/4"	8"	6250#	110 #-FT



3. PLACEMENT GUIDELINES FOR ABOVE VALUES IN ITEM 2 REQUIRE THE FOLLOWING CONDITIONS:
   - TABLE VALUES ARE BASED ON fc = 3000 PSI
   - HOLES DRILLED WITH A HAMMER DRILL AND CARBIDE BIT COMPLYING W/ ANSI B212.15-1994
   - BIT DIAMETER EQUALS THE SIZE OF THE ANCHOR BEING INSTALLED
   - HOLE DEPTH MUST EXCEED MIN. EMBED. BY ONE BOLT DIAMETER
   - ANCHOR SPACING IS THE GREATER OF (1.5 * FLUTE WIDTH) OR (3 * EMBEDMENT)
   - ANY SEISMIC DESIGN CATEGORY PER I.B.C. 2009
   - TENSION LOAD VALUES SHALL BE MULTIPLIED BY 0.6 FOR LIGHTWEIGHT CONCRETE
   - A.C.I. "CRACKED" CONCRETE CONDITION IS SUFFICIENT
   - CARBON BOLTS ONLY
   - ONLY 3/8" DIA. OR 1/2" DIA. ANCHORS MAY BE INSTALLED IN THE TOPSIDE OF CONCRETE OVER METAL DECK ASSEMBLIES WITH A 2" MAXIMUM EMBEDMENT.

4. WHEN INSTALLING EXPANSION ANCHORS IN EXISTING CONCRETE, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS. MAINTAIN A MINIMUM CLEARANCE OF ONE-INCH BETWEEN THE EXISTING REINFORCEMENT AND THE EXPANSION ANCHOR.

5. ANY BOLTS SHOWN ON THE APPROVED PLANS AS EXPANSION ANCHORS, REQUIRES SPECIAL INSPECTION IN ACCORDANCE WITH SECTION 4.4 IN THE I.C.C. REPORT. SPECIAL INSPECTION SHALL BE BY AN APPROVED TESTING AND INSPECTION AGENCY. ANY ITEMS THAT REQUIRE EXPANSION ANCHORS BUT ARE NOT SPECIFICALLY SHOWN ON THE APPROVED PLANS MUST BE APPROVED BY THE STRUCTURAL ENGINEER AND BUILDING DEPARTMENT PRIOR TO INSTALLATION.

D1 HILTI KWIK BOLT TZ IN CONC. OVER MTL DECKS

SS101 NOT TO SCALE

HILTI OR EQUIVALENT

1. EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ AS MANUFACTURED BY HILTI, INC., 5400 SOUTH 122ND EAST AVENUE, TULSA, OKLAHOMA 74146. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND I.C.C. REPORT NO. ESR-1917.

2. ULTIMATE TENSION VALUES SHALL BE AS FOLLOWS:

BOLT SIZE	MIN. EMBED	EDGE DIST.	SPACING	CONT. THKS.	TENSION LOAD	TORQUE TEST
3/8"	2"	4 1/2"	5"	4"	1600#	25 #-FT
1/2"	3 1/4"	7 3/4"	6"	6"	3475#	40 #-FT
5/8"	4"	9"	6"	6"	4150#	60 #-FT
3/4"	4 3/4"	10 1/4"	9"	9"	6250#	110 #-FT

3. PLACEMENT GUIDELINES FOR ABOVE VALUES IN ITEM 2 REQUIRE THE FOLLOWING CONDITIONS:
   - TABLE VALUES ARE BASED ON fc = 3000 PSI
   - HOLES DRILLED WITH A HAMMER DRILL AND CARBIDE BIT COMPLYING W/ ANSI B212.15-1994
   - BIT DIAMETER EQUALS THE SIZE OF THE ANCHOR BEING INSTALLED
   - HOLE DEPTH MUST EXCEED MIN. EMBED. BY ONE BOLT DIAMETER
   - ANY SEISMIC DESIGN CATEGORY PER I.B.C. 2009
   - TENSION LOAD VALUES SHALL BE MULTIPLIED BY 0.6 FOR LIGHTWEIGHT CONCRETE
   - A.C.I. "CRACKED" CONCRETE CONDITION IS SUFFICIENT
   - FOR CARBON OR STAINLESS STEEL BOLTS

4. WHEN INSTALLING EXPANSION ANCHORS IN EXISTING CONCRETE, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS. MAINTAIN A MINIMUM CLEARANCE OF ONE-INCH BETWEEN THE EXISTING REINFORCEMENT AND THE EXPANSION ANCHOR.

5. ANY BOLTS SHOWN ON THE APPROVED PLANS AS EXPANSION ANCHORS, REQUIRES SPECIAL INSPECTION IN ACCORDANCE WITH SECTION 4.4 IN THE I.C.C. REPORT. SPECIAL INSPECTION SHALL BE BY AN APPROVED TESTING AND INSPECTION AGENCY. ANY ITEMS THAT REQUIRE EXPANSION ANCHORS BUT ARE NOT SPECIFICALLY SHOWN ON THE APPROVED PLANS MUST BE APPROVED BY THE STRUCTURAL ENGINEER AND BUILDING DEPARTMENT PRIOR TO INSTALLATION.

D2 HILTI KWIK BOLT TZ IN CONC.

SS101 NOT TO SCALE

HILTI OR EQUIVALENT

1. THESE NOTES GOVERN ALL CONDITIONS CALLED OUT ON THE PLANS AS "SHOT PINS" UNLESS SPECIFICALLY NOTED OTHERWISE.

2. ALL SHOT PINS SHALL BE AS MANUFACTURED BY HILTI, INCORP. REFERENCE SHALL BE MADE TO THE LATEST EDITION OF THE HILTI 'PRODUCT TECHNICAL GUIDE' AND THE ICC-ES ESR-2269 REPORT FOR ADDITIONAL INFORMATION.

3. SHOT PINS DRIVEN INTO STEEL BASE MATERIAL SHALL BE X-U TYPE WITH P8 WASHERS. LENGTH OF PIN SHALL BE AS REQUIRED TO PENETRATE THROUGH THE STEEL BASE MATERIAL. MINIMUM EDGE DISTANCE TO ANY CONNECTED PART SHALL BE 1/2" AND MINIMUM FASTENER SPACING SHALL BE 2". ENTIRE POINTED END OF PIN MUST PENETRATE THROUGH STEEL LESS THAN 1/2" THICK OR PENETRATE A MINIMUM OF 1/2" INTO STEEL 1/2" THICK OR GREATER. PINS IN STEEL SUBJECT TO WITHDRAW LOADS ARE REQUIRED TO HAVE KNURLED SHANK.

4. SHOT PINS DRIVEN INTO CONCRETE BASE MATERIAL SHALL BE X-U TYPE WITH P8 WASHERS. LENGTH OF PIN SHALL BE AS REQUIRED TO PENETRATE 1 1/2" INTO THE CONCRETE BASE MATERIAL. MINIMUM EDGE DISTANCE TO ANY CONCRETE MATERIAL SHALL BE 3" AND MINIMUM FASTENER SPACING SHALL BE 4".

5. SHOT PINS DRIVEN INTO CONCRETE BASE MATERIAL THROUGH METAL DECK SHALL BE X-U TYPE WITH P8 WASHERS. LENGTH OF PIN SHALL BE AS REQUIRED TO PENETRATE 1" INTO THE CONCRETE THROUGH THE LOW FLUTE. PIN SHALL BE CENTERED IN THE LOW FLUTE AND MINIMUM FASTENER SPACING SHALL BE 4".

6. WHERE STEEL WASHERS ARE INDICATED ON THE DRAWINGS, PINS SHALL BE X-U WITH PREMOUNTED STEEL WASHERS WITH A MINIMUM DIAMETER OF 36mm (1 7/16").

7. PDP SHALL NOT BE USED TO RESIST SEISMIC OR SHORT TERM LATERAL LOADS

B4 "SHOT PIN" NOTES FOR HILTI X-U

SS101 NOT TO SCALE

A.B.	ANCHOR BOLT	HSS	HOLLOW STRUCTURAL SECTION
ABV.	ABOVE	HT.	HEIGHT
A.C.I.	AMERICAN CONCRETE INSTITUTE	I.B.C.	INTERNATIONAL BUILDING CODE
ADDL.	ADDITIONAL	I.C.C.	INTERNATIONAL CODE COUNCIL
A.E.S.A.	ARCHITECTURAL EXPOSED STEEL	IN	INCH
A.F.P.A.	AMERICAN FOREST AND PAPER ASSOCIATION	I.D.	INSIDE DIAMETER
A.I.S.C.	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	INT.	INTERIOR
A.I.T.C.	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION	I.R.	INTERPRETATION OF REGULATIONS
ALT.	ALTERNATE	JST.	JOIST
A.P.A.	AMERICAN PLYWOOD ASSOCIATION	K	KIPS
ARCH.	ARCHITECT(URAL)	KSI	KIPS PER SQUARE INCH
A.E.S.S.	ARCHITECTURAL EXPOSED STRUCTURAL STEEL	L	ANGLE
A.S.C.E.	AMERICAN SOCIETY OF CIVIL ENGINEERS	LBS, #	POUNDS (XXX LBS, XXX#)
A.S.T.M.	AMERICAN SOCIETY FOR TESTING & MATERIALS	LL	LIVE LOAD
A.W.S.	AMERICAN WELDING SOCIETY	LLV(LLH)	LONG LEG VERTICAL (HORIZ.)
BLDG.	BUILDING	LOC.	LOCATION
BLK., BLKG.	BLOCK, BLOCKING	LSL	LAMINATED STRAND LUMBER
BM.	BEAM	LT. WT.	LIGHT WEIGHT
B.O.	BOTTOM OF	LVL	LAMINATED VENEER LUMBER
BOT.	BOTTOM	MAS.	MASONRY
C	CHANNEL	MAX.	MAXIMUM
C.B.C.	CALIFORNIA BUILDING CODE	M.B.	MACHINE BOLT
CDX	C-D EXPOSURE 1	MFR	MANUFACTURER
C.J.	CONSTRUCTION JOINT	MIN.	MINIMUM
CL	CENTERLINE	M.S.R.	MACHINE STRESS RATED
CLG.	CEILING	MTL.	METAL
CLR.	CLEAR	(N)	NEW
C.M.U.	CONCRETE MASONRY UNIT	NO., #	NUMBER, (NO. XX, #XX)
COL.	COLUMN	N.T.S.	NOT TO SCALE
CONC.	CONCRETE	O/, o/	OVER
CONN.	CONNECTION	O.C.	ON CENTER
CONT.	CONTINUOUS	O.D.	OUTSIDE DIAMETER
CTSK.	COUNTERSINK	O.H.	OPPOSITE HAND
d	PENNY NAILS	OPNG.	OPENING
DBL.	DOUBLE	OP.	OPPOSITE
DEMO.	DEMOLISH	OSB	ORIENTED STRAND BOARD
DET., DTL.	DETAIL	O.S.H.P.D.	OFFICE OF STATEWIDE HEALTH PLANNING AND DEVELOPMENT
DF	DOUGLAS FIR	PL.	PLATE
DIA., Ø	DIAMETER	P.T.	PRESSURE TREATED
DIAG.	DIAGONAL	PLYWD.	PLYWOOD
DIM.	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DL	DEAD LOAD	PSI	POUNDS PER SQUARE INCH
DO	DITTO / DO OVER	REINF.	REINFORCING
DP	DEEP	REQD.	REQUIRED
D.S.A.	DIVISION OF STATE ARCHITECT	RM.	ROOM
DWG	DRAWING	SCHED.	SCHEDULE
(E)	EXISTING	SHTG.	SHEATHING
EA.	EACH	SHT.	SHEET
ELEV.	ELEVATION	SIM.	SIMILAR
E.N.	EDGE NAILING	SMS	SHEET METAL SCREWS
ENGR.	ENGINEER	SPEC.	SPECIFICATION
EQ.	EQUAL	SQ.	SQUARE
EQUIP.	EQUIPMENT	STGR.	STAGGER
EXP.	EXPANSION	STD.	STANDARD
E.W.	EACH WAY	STIFF.	STIFFENER
FDN.	FOUNDATION	STL.	STEEL
F.E.M.A.	FEDERAL EMERGENCY MANAGEMENT AGENCY	STRUCT.	STRUCTURAL
F.F.	FINISH FLOOR	SYM.	SYMMETRICAL
FIN.	FINISH	T&B	TOP AND BOTTOM
FLR.	FLOOR	T&G	TONGUE AND GROOVE
F.N.	FIELD NAILING	THK.	THICK
FRMG.	FRAMING	TL	TOTAL LOAD
FT., FTG.	FOOT, FOOTING	T.O.	TOP OF
F.V.	FIELD VERIFY	TYP.	TYPICAL
GA.	GAUGE	U.N.O., U.O.N.	UNLESS NOTED OTHERWISE
GALV.	GALVANIZED	VERT.	VERTICAL
GLB	GLUE LAMINATED BEAM	W/, W/O	WITH, WITH OUT
H.D.	HOLDOWN	WD	WOOD
HDR.	HEADER	W.F.	WIDE FLANGE
HGR.	HANGER	W.C.L.I.B.	WEST COAST LUMBER INSPECTION BUREAU
HORIZ.	HORIZONTAL	WT.	WEIGHT
		W.W.F.	WELDED WIRE FABRIC
		W.W.A.	WELDED WIRE MESH
		W.W.P.A.	WESTERN WOOD PRODUCTS ASSOCIATION

B6 GENERAL PROJECT NOTES

SS101 NOT TO SCALE

1. ANY SUBSTITUTIONS FOR STRUCTURAL ITEMS ON APPROVED PLANS SHALL BE REVIEWED BY THE DEPARTMENT OF VETERANS AFFAIRS PRIOR TO USE. NO GUARANTEE THE SUBSTITUTION WILL BE ALLOWED.

2. DETAILS AND NOTES ON TYPICAL SHEETS SHALL APPLY U.N.O. DETAILS OF CONSTRUCTION NOT FULLY SHOWN SHALL BE OF THE SAME NATURE AS SHOWN FOR SIMILAR CONDITIONS. U.N.O.

3. DO NOT SCALE STRUCTURAL DRAWINGS. IF DIMENSIONS IN DRAWINGS ARE NOT CLEAR, OR DISCREPANCIES EXIST ON THE DRAWINGS OR SPECIFICATIONS, CONTACT THE ENGINEER.

4. SEE MECHANICAL, ELECTRICAL, AND/OR ARCHITECTURAL DRAWINGS FOR LOCATION AND SIZE OF PIPES, CONDUITS, FLOOR DRAINS, VENTS, DUCTS, DRAIN LEADERS AND OTHER SIMILAR OPENINGS NOT INDICATED ON THE STRUCTURAL DRAWINGS.

5. SEE MECHANICAL, ELECTRICAL AND/OR ARCHITECTURAL DRAWINGS FOR EMBEDMENT OF BOLTS, ANCHORS AND OTHER MISCELLANEOUS EMBEDDED ITEMS NOT SHOWN ON STRUCTURAL DRAWINGS. BOLT LOCATIONS AND ELEVATIONS SHALL BE CERTIFIED BY LICENSED SURVEYOR PRIOR TO PLACEMENT OF CONCRETE.

6. THE DESIGN, FABRICATION AND CONSTRUCTION SHALL COMPLY WITH ACCEPTED LOCAL GOVERNING CODES OF THE PARTICULAR AREA.

7. THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. UNLESS OTHERWISE SHOWN, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL SOLELY AND COMPLETELY BE RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.

8. THE DUTY OF THE ENGINEER TO CONDUCT CONSTRUCTION REVIEW OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES IN, ON, OR NEAR THE CONSTRUCTION SITE.

9. CONSTRUCTION SHALL BE DISTINGUISHED FROM CONTINUOUS AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ENGINEER WHETHER PERFORMED PRIOR TO, DURING, OR AFTER COMPLETION OF CONSTRUCTION ARE PERFORMED SOLELY FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DRAWINGS AND SPECIFICATIONS, BUT THEY DO NOT GUARANTEE CONTRACTOR'S PERFORMANCE AND SHALL NOT BE CONSTRUED AS SUPERVISION OF CONSTRUCTION.

10. ALL WORK SHALL CONFORM TO THE LATEST APPLICABLE CONSTRUCTION SAFETY REQUIREMENTS OF O.S.H.A. AND ANY OTHER GOVERNMENTAL ENTITY HAVING JURISDICTION.

11. SHOP DRAWINGS:
    - SHOP DRAWINGS SHALL BE SUBMITTED PER PROJECT SPECIFICATIONS.
    - PRIOR TO SUBMISSION, THE CONTRACTOR SHALL REVIEW ALL SUBMITTALS FOR CONFORMANCE WITH THE APPROVED DOCUMENTS AND SHALL STAMP SUBMITTALS AS BEING "REVIEWED FOR CONFORMANCE" TO APPROVED CONTRACT DOCUMENTS.
    - THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE DESIGN TEAM THAT HE AND THE SUBCONTRACTOR UNDERSTAND THE DESIGN CONCEPT BY INDICATING WHICH MATERIAL HE INTENDS TO FURNISH AND INSTALL AND BY DETAILING THE FABRICATION AND INSTALLATION METHODS HE INTENDS TO USE.
    - DESIGN DOCUMENTS ARE NOT SHOP DRAWINGS AND SHALL NOT BE SUBMITTED AS SUCH.

12. ALL NOTES SPECIFIED ON PLANS AND IN DETAILS WITH THE WORD "TYPICAL" FOLLOWED BY **BOLD AND UNDERLINED** TEXT REFER TO THE TYPICAL PROJECT DETAILS ON SS100 SERIES SHEETS. TYPICAL DETAILS ARE NOT SPECIFICALLY REFERENCED ON PLANS AND SPECIFIC DETAIL U.N.O.

13. ANY DEFECTIVE OR DEFICIENT WORK OR WORK THAT IS IN NON-COMPLIANCE TO THE APPROVED PLANS, SHALL BE REMOVED AND REPLACED PER APPROVED PLANS, AT NO ADDITIONAL COST TO THE GOVERNMENT.

14. PRIOR TO PLACING CONCRETE, ALL ANCHOR BOLT LOCATIONS AND ELEVATIONS SHALL BE SURVEYED AND SUBMITTED AS A SHOP DRAWING SUBMITTAL.

D8 DESIGN CRITERIA

SS101 NOT TO SCALE

CODE
- 2009 INTERNATIONAL BUILDING CODE
- AMERICAN SOCIETY OF CIVIL ENGINEERS (ASCE 7-05)
- (UFC 3-300-10N)
- (UFC 3-310-01)
- (UFC 4-010-01)

OCCUPANCY CATEGORY: TYPE II

ROOF (W/ VERCO DECK) LOAD
- DEAD LOADS (INCLUDES 5 PSF FOR PV PANELS): 70 PSF
- LIVE LOAD: 20 PSF

ROOF (AT MECHANICAL WELL) LOAD
- DEAD LOADS: 94 PSF
- LIVE LOAD: 20 PSF

ROOF (W/ EPICORE DECK) LOAD
- DEAD LOADS (INCLUDES 5 PSF FOR PV PANELS): 80 PSF
- LIVE LOAD: 20 PSF

FLOOR LOADS
- DEAD LOADS: 95 PSF
- LIVE LOADS: 100 PSF

WALL LOADS
- EXTERIOR WALL DEAD LOADS: 16 PSF
  (20 PSF PARTITION LOAD INCLUDED IN FLOOR DEAD LOAD)

SNOW LOADS
- SNOW IMPORTANCE FACTOR: 1.0
- GROUND SNOW LOADS: 0 PSF
- FLAT ROOF SNOW LOADS: 0 PSF
- SLOPED ROOF SNOW LOADS: 0 PSF

SEISMIC LOADING CRITERIA
- SEISMIC IMPORTANCE FACTOR: 1
- MAPPED SPECTRAL ACCELERATION, MCE:
  - a. Ss: 0.506
  - b. S1: 0.223
- SPECTRAL RESPONSE COEFFICIENT:
  - a. Fa: 1.395
  - b. Fv: 1.955
- MAXIMUM CONSIDERED EARTHQUAKE RESPONSE ACCELERATIONS:
  - a. Sms: 0.699
  - b. Sm1: 0.434
- DESIGN SPECTRAL RESPONSE ACCELERATIONS:
  - a. Sds: 0.471
  - b. Sd1: 0.291
- SEISMIC DESIGN CATEGORY: D
- RESPONSE MODIFICATION COEFFICIENT, R (FRAMING TYPE): 8
- SEISMIC RESPONSE COEFFICIENT/BASE SHEAR: 0.058 *W
- SEISMIC SYSTEM OVERSTRENGTH FACTOR: 3
- SEISMIC DEFLECTION AMPLIFICATION FACTOR: 5.5
- SEISMIC FORCE REDUNDANCY FACTOR: 1.0

WIND LOADING CRITERIA:
- IMPORTANCE FACTOR: 1.00
- BASIC WIND SPEED: 85 MPH
- SURFACE ROUGHNESS CATEGORY: B
- EXPOSURE CATEGORY: B
- VELOCITY PRESSURE EXPOSURE COEFFICIENT, Kz: 0.7
- TOPOGRAPHIC FACTOR, Kzt: 1.00
- ENCLOSURE CLASSIFICATION: ENCLOSED
- ROOF SLOPE: <2:12
- WORST CASE MAIN WIND FORCE RESISTING SYSTEM
  - DESIGN WALL WIND LOADS, Pnet: 12.2 PSF
  - DESIGN PARAPET WIND LOADS, Pnet: 48.8 PSF
  - DESIGN UPLIFT WIND LOADS, Pnet: 14.1 PSF
- WORST CASE COMPONENTS AND CLADDING
  - PRESSURE COEFFICIENTS WALLS, Cnet: 1.34
  - PRESSURE COEFFICIENTS PARAPETS, Cnet: 3.64

SOILS CRITERIA:
- ALLOWABLE BEARING PRESSURE (DL+LL): 3000 PSF
- PASSIVE EARTH PRESSURE: 280 PSF
- COEFFICIENT OF FRICTION: 0.4
- SITE CLASSIFICATION: C
- TESTING LAB: BSK ASSOCIATES
- SOILS REPORT NUMBER: BSK G11-143-11F
- SOILS REPORT DATE: JANUARY 5, 2012

PHYSICAL SECURITY CRITERIA:
- DESIGN BASIS: PHYSICAL SECURITY DESIGN MANUAL; LIFE SAFETY PROTECTED
- APPLIED LOAD: W1 & GP1
- STAND-OFF DISTANCE: 25 FT

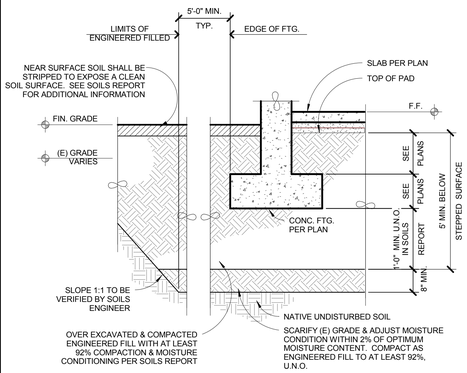
ANCHORAGE OF ALL EQUIPMENT TO BE INSTALLED, AS A PART OF THIS PROJECT SHALL BE DETAILED ON THESE PLANS, EXCEPT FOR THE FOLLOWING:

1. EQUIPMENT WEIGHING LESS THAN 400 POUNDS SUPPORTED DIRECTLY ON THE FLOOR OR ROOF.
2. FURNITURE (NON FIXED AND MOVABLE AS EXEMPTED BY I.B.C. 2009 CHAPTER 1 SECTION 105.2).
3. TEMPORARY OR MOVABLE EQUIPMENT NON FIXED AND MOVABLE AS EXEMPTED BY THE I.B.C. 2009 CHAPTER 1 SECTION 105.2).
4. EQUIPMENT WEIGHING LESS THAN 20 POUNDS SUPPORTED BY VIBRATION ISOLATORS.
5. EQUIPMENT WEIGHING LESS THAN 20 POUNDS SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM A WALL.

PERMANENT EQUIPMENT IN ITEMS 1, 4, AND 5 MUST BE SUPPORTED AND ANCHORED TO RESIST THE FORCED THE PRESCRIBED BY CHAPTER 13 OR ASCE 7 AS MODIFIED BY THE I.B.C. 2009 SECTIONS 1613A/1514A AND THE ANCHORAGE SHALL BE APPROVED BY THE APPROPRIATE DESIGN PROFESSIONAL OF RECORD AND DEPARTMENT OF VETERANS AFFAIRS, AS A PART OF FIELD REVIEWS/OBSERVATIONS. THE INSPECTOR OF RECORD SHALL ASSURE THAT THE ABOVE REQUIREMENTS ARE ENFORCED.

F1 EQUIPMT. ANCHORAGE REQ'MT.

SS101 NOT TO SCALE



F2 OVER EXCAVATION

SS101 NOT TO SCALE

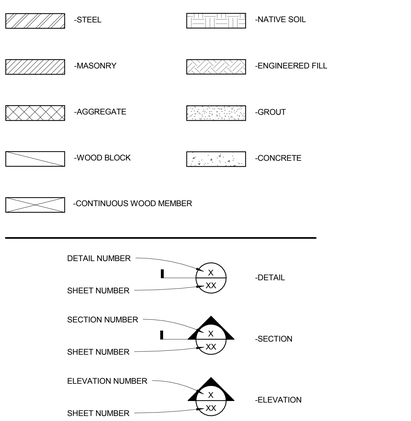
VERIFICATION AND INSPECTION TASK		FREQUENCY OF INSPECTION	REQUIRED FOR PROJECT
1.	VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	PERIODIC	✓
2.	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	PERIODIC	✓
3.	PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS	PERIODIC	✓
4.	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL	CONTINUOUS	✓
5.	PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	PERIODIC	✓

NOTE: ALL INSPECTIONS REQUIRED BY GEOTECHNICAL REPORT ARE ALSO REQUIRED.

SEE TYPICAL SHEET **SS103** FOR CONCRETE AND STEEL SPECIAL INSPECTION NOTES

F6 SPECIAL INSPECTION FOR SOIL

SS101 NOT TO SCALE



F8 LEGENDS AND SYMBOLS

SS101 NOT TO SCALE

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Drawing Title: TYPICAL NOTES AND DETAILS

Approved: Project Director

Project Title: Mental Health Psychosocial Rehab Recovery and Health Care for Homeless Veteran Center

Location: 2615 E. CLINTON AVE, FRESNO, CA 93703

Date: 08/11/2014 | Checked: AL | Drawn: ESB

Project Number: 570-217

Building Number: 37

Drawing Number: SS101

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Office of Construction and Facilities Management:

Department of Veterans Affairs